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GENERAL STRUCTURAL NOTES

GOVERNING CODE

2008 INDIANA BUILDING CODE

DESIGN LOADS

1. CLEARSTORY ROOF LOAD:

A.	MINIMUM LIVE LOAD OR SNOW LOAD (Pf)	20	PSF
B.	STANDING SEAM ROOF DECK	2	PSF
C.	¾" PLYWOOD	3	PSF
D.	ROOF JOIST	2	PSF
E.	INSULATION	3	PSF
F.	CEILING	3	PSF
G.	SPRINKLERS, MISC. MECHANICAL	5	PSF
H.	LIGHTS, ELECTRIC	3	PSF
TOTAL LOAD ON JOISTS		41	PSF
I.	BEAMS	4	PSF
TOTAL ON BEAMS		45	PSF

2. MAIN ROOF LOAD:

A.	MINIMUM LIVE LOAD OR SNOW LOAD (Pf)	20	PSF
B.	MEMBRANE ROOF SYSTEM	2	PSF
C.	INSULATION	6	PSF
D.	¾" PLYWOOD	3	PSF
E.	ROOF JOIST	2	PSF
F.	CEILING	3	PSF
G.	SPRINKLERS, MISC. MECHANICAL	5	PSF
H.	LIGHTS, ELECTRIC	3	PSF
TOTAL LOAD ON JOISTS		44	PSF
I.	BEAMS	4	PSF
TOTAL ON BEAMS		48	PSF

3. CANOPY ROOF LOAD:

A.	MINIMUM LIVE LOAD OR SNOW LOAD (Pf)	20	PSF
B.	MEMBRANE ROOF SYSTEM	2	PSF
C.	¾" PLYWOOD	3	PSF
D.	ROOF JOIST	2	PSF
E.	CEILING	3	PSF
F.	SPRINKLERS, MISC. MECHANICAL, LIGHTS	4	PSF
TOTAL LOAD ON JOISTS		34	PSF
G.	BEAMS	2	PSF
TOTAL ON BEAMS		36	PSF

4. MAIN BUILDING SNOW LOAD:

A.	FLAT ROOF SNOW LOAD WITH DRIFT MODIFIED BY APPLICABLE DRIFT COEFFICIENTS	Pf = 14 PSF
B.	SNOW EXPOSURE FACTOR	Ce = 1.0
C.	SNOW LOAD IMPORTANCE FACTOR	I = 1.0
D.	THERMAL FACTOR	Ct = 1.0

5. DRIVE THRU CANOPY SNOW LOAD:

A.	FLAT ROOF SNOW LOAD	Pf = 14 PSF
B.	SNOW EXPOSURE FACTOR	Ce = 1.0
C.	SNOW LOAD IMPORTANCE FACTOR	I = 1.0
D.	THERMAL FACTOR	Ct = 1.2

6. WIND LOAD:

A.	BASIC WIND SPEED	90 MPH 3-SECOND GUST
B.	WIND LOAD IMPORTANCE FACTOR	I = 1.00
C.	WIND EXPOSURE	B
D.	INTERNAL PRESSURE COEFFICIENT	GCp = 0.18, ENCLOSED BUILDING

7. SEISMIC LOAD:

A.	SEISMIC IMPORTANCE FACTOR	I = 1.0
B.	OCCUPANCY CATEGORY	II
C.	MAPPED SPECTRAL RESPONSE ACCELERATION,	Ss = 0.169g
		Sl = 0.067g
D.	SPECTRAL RESPONSE COEFFICIENT,	Sds = 0.135g
		Sdl = 0.076g
E.	SITE CLASS	C
F.	SEISMIC DESIGN CATEGORY	SDC = B
G.	BASIC SEISMIC FORCE RESISTING SYSTEM: LIGHT-FRAMED WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE	
H.	DESIGN BASE SHEAR	V = 3.1 KIPS
I.	SEISMIC RESPONSE COEFFICIENT	Cs =0.021
J.	RESPONSE MODIFICATION FACTOR	R= 6.5
K.	ANALYSIS PROCEDURE	ELFP

CONSTRUCTION AND SAFETY

1. CONTRACTOR SHALL BRACE ENTIRE STRUCTURE AS REQUIRED TO MAINTAIN STABILITY UNTIL COMPLETE AND FUNCTIONING AS THE DESIGNED UNIT.

2. ENGINEER SHALL NOT BE RESPONSIBLE FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES OF CONSTRUCTION SELECTED BY CONTRACTOR.

3. THE CONTRACTOR WILL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS. WHEN ON SITE, THE ENGINEER IS RESPONSIBLE FOR HIS OWN SAFETY BUT HAS NO RESPONSIBILITY FOR THE SAFETY OF OTHER PERSONNEL OR SAFETY CONDITIONS AT THE SITE.

FOUNDATIONS

1. FOUNDATION DESIGN IS BASED ON RECOMMENDATIONS DESCRIBED IN THE "GEOTECHNICAL EXPLORATION REPORT, FIRST FINANCIAL BANK, SHELBYVILLE, SHELBY COUNTY, INDIANA" BY ATC ASSOCIATES, INC., DATED DECEMBER 22, 2009.

2. BEARING STRATA SHALL BE APPROVED BY A GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE. FOLLOW THE GEOTECHNICAL REPORT RECOMMENDATIONS FOR OVER-EXCAVATION UNDER THE FOOTINGS. PROVIDE ENGINEERED FILL OR FLOWABLE FILL CONCRETE (500 PSI) UNDER FOUNDATIONS. INSTALL FOUNDATIONS AT DESIGNED ELEVATIONS.

3. ALL FOOTINGS SHALL BEAR ON LEVEL (WITHIN 1 IN 12) APPROVED ENGINEERED FILL. FOUNDATIONS HAVE BEEN DESIGNED FOR A MAXIMUM SOIL BEARING PRESSURE OF 2500 PSF BELOW STRIP FOOTINGS AND 2500 PSF BELOW ISOLATED COLUMN FOOTINGS.

4. CONTRACTOR SHALL CONTACT UTILITY COMPANIES FOR LOCATING UNDERGROUND SERVICES AND IS RESPONSIBLE FOR THEIR PROTECTION AND SUPPORT.

5. COMPACTION:

   A. ALL FILL MATERIALS SHALL BE APPROVED BY A GEOTECHNICAL CONSULTANT.

   B. SEE GEOTECHNICAL REPORT FOR FILL REQUIREMENTS (IF REQUIRED) FOR FOOTINGS AND SLAB ON GRADE SUPPORT.

6. ALL AREAS WITHIN THE FOOTPRINT OF THE BUILDING, INCLUDING UTILITY TRENCHES, MUST BE FREE OF ANY WET AND/OR SOFT AREAS PRIOR TO PLACEMENT OF FILL MATERIAL OR SLAB.

7. SEAL UTILITY TRENCH AT THE EXTERIOR FOUNDATION WALL BY USING A COMPACTED CLAYEY BACKFILL OR LEAN CONCRETE TO CREATE A DAM TO PREVENT ENTRY OF WATER.

8. FINISHED GRADE SHALL SLOPE AWAY FROM THE PERIMETER FOUNDATION.

CONCRETE

1. CONCRETE WORK AND TESTING SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS", EXCEPT AS MODIFIED BY THE SUPPLEMENTAL REQUIREMENTS BELOW. REPORTS FROM TESTS REQUIRED BY SECTION 1.6 OF ACI 301 SHALL BE SUBMITTED TO STRUCTURAL ENGINEER, ARCHITECT, OWNER, CONTRACTOR, CONCRETE SUPPLIER, AND BUILDING OFFICIAL.

2. CONCRETE WORK IN COLD WEATHER SHALL CONFORM TO ALL REQUIREMENTS OF ACI 306.1 "STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING" AND ACI 306R "COLD WEATHER CONCRETING".

3. CONCRETE WORK IN HOT WEATHER SHALL CONFORM TO ALL REQUIREMENTS OF ACI 305R "HOT WEATHER CONCRETING". THE AIR TEMPERATURE, RELATIVE HUMIDITY, CONCRETE TEMPERATURE, AND WIND VELOCITY SHALL BE ENTERED INTO THE NOMOGRAPH OF THIS REFERENCE TO DETERMINE IF PRECAUTIONS AGAINST PLASTIC SHRINKAGE ARE REQUIRED.

4. CONCRETE MIX DESIGNS SHALL BE SUBMITTED FOR EACH TYPE OF CONCRETE TO THE STRUCTURAL ENGINEER FOR APPROVAL IN ACCORDANCE WITH ACI 301 SECTION 4.2.3.4 FIELD TEST DATA OR TRIAL MIXTURES.

5. SUBMIT SHOP DRAWINGS OF REINFORCING STEEL.

6. MATERIALS: (f'c BASED ON 28 DAY UNLESS NOTED)

   A. CONCRETE UNLESS NOTED: f'c = 4000 PSI., NORMAL AGGREGATE.

   B. CONCRETE FOR INTERIOR FLOOR SLABS: f'c = 4000 PSI AT 28 DAYS, NORMAL WEIGHT AGGREGATE, MINIMUM PORTLAND CEMENT CONTENT PER ACI 301 TABLE 4.2.2.1, WATER NOT PERMITTED TO BE ADDED AT THE SITE, MAXIMUM WATER/CEMENTITIOUS RATIO = 0.50.

   C. CONCRETE FOR EXTERIOR FLAT WORK, WALKS, ETC.: f'c = 4500 PSI, (4.5% TO 7.5% ENTRAINED AIR), MINIMUM PORTLAND CEMENT CONTENT = 520 #/CY, MAXIMUM WATER/CEMENTITIOUS RATIO = 0.45.

   D. CONCRETE FOR FOOTINGS: f'c = 3000 PSI.

   E. REINFORCING STEEL: ASTM A615 60 KSI YIELD DEFORMED BARS AND ASTM A185 MESH, FLAT SHEETS ONLY.

   F. FLY ASH: ASTM C618, TYPE F OR C. FLY ASH-TO- TOTAL CEMENTITIOUS RATIO SHALL NOT EXCEED 25% MAXIMUM.

   G. GROUND GRANULATED BLAST FURNACE SLAG: ASTM C989. TOTAL GROUND GRANULATED BLAST FURNACE SLAG -TO- TOTAL CEMENTITIOUS RATIO SHALL NOT EXCEED 50% MAXIMUM.

   H. HIGH RANGE WATER REDUCER (HRWR) ADMIXTURE: ASTM C494.

   I. CHLORIDE CONTENT OF CONCRETE: LIMIT TOTAL CHLORIDE ION CONTENT TO AMOUNT INDICATED IN TABLE 4.2.2.6 OF ACI 318. ADMIXTURES CONTAINING CHLORIDE ARE NOT PERMITTED IN REINFORCED CONCRETE OR CONCRETE CONTAINING METALS.

7. SLUMP SHALL BE MEASURED PRIOR TO THE ADDITION OF HRWR.

8. LAP SPLICE REINFORCING BARS 48 BAR DIAMETERS UNLESS NOTED OTHERWISE.

9. BAR CLEARANCES BETWEEN ADJACENT BARS AND FORMWORK SHALL BE AS NOTED ON THE DRAWINGS OR A MINIMUM AS PER ACI REQUIREMENTS.

10. AT CORNERS AND INTERSECTIONS OF FOOTINGS PROVIDE BENT BARS OF EQUAL SIZE AND AT SAME SPACING AS TYPICAL REINFORCING AROUND CORNER. BARS SHALL HAVE EMBEDMENT OF 30 DIAMETERS (18" MIN.).

11. MACHINE TROWEL FINISH FLOOR SLAB AND CURE USING "CURE AND SEAL" TYPE CURING COMPOUND MEETING FEDERAL SPECIFICATION TT-C-00800, VOC COMPLIANT, 30% MINIMUM SOLIDS CONTENT. FOR APPLICATIONS EXPOSED TO SUNLIGHT USE LIGHT BROOM FINISH AND ACRYLIC BASED CURING COMPOUND.

12. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR VAPOR BARRIER REQUIREMENTS. VAPOR BARRIER, WHERE REQUIRED, SHALL BE PLACED OVER COMPACTED GRANULAR SUBBASE.

13. REINFORCE ALL INTERIOR SLABS ON GROUND WITH 6 X 6 - W2.9 X W2.9 (42#) MESH. LOCATE MESH 2" CLEAR BELOW TOP OF SLAB.

14. LAP WELDED WIRE FABRIC MINIMUM 1 FULL SPACE PLUS 2".

15. FINISH OF CONCRETE HANDICAP RAMPS TO CONFORM TO THE REQUIREMENTS OF THE AMERICANS WITH DISABILITIES ACT (ADA). COORDINATE LOCATION AND PATTERN WITH ARCHITECTURAL DRAWINGS.

16. CONTROL JOINTS IN SLABS ON GROUND SHALL BE LOCATED AT 12'-0" MAXIMUM SPACING AND SHALL CREATE SECTIONS OF SLAB WITH A MAXIMUM ASPECT RATIO OF 1½ TO 1. CONTROL JOINTS SHALL BE SAWN AND SHALL BE A MINIMUM OF ¼ OF THE SLAB THICKNESS DEEP. THE CONTROL JOINT SHALL BE SAWN AS SOON AS THE SAW BLADE CAN CUT THE CONCRETE WITHOUT DISPLACING THE AGGREGATE. CUT EVERY OTHER MESH WIRE AT THE CONTROL JOINT LOCATION PRIOR TO PLACING CONCRETE.

17. PROVIDE ¾" CHAMFER AT CORNERS OF EXPOSED CONCRETE.

18. WHERE BRITTLE FLOOR FINISHES ARE TO BE APPLIED TO FLOOR SLABS, COORDINATE CONTROL JOINT LOCATIONS WITH FLOOR FINISH JOINT LOCATIONS AND ARCHITECT.

EXPANSION ANCHORS

1. EXPANSION ANCHORS SHALL BE MANUFACTURED BY ITW Ramset/RedHead AND SHALL BE THE TYPE, SIZE, AND EMBEDMENT INDICATED ON DRAWINGS. INSTALL PER MANUFACTURER'S RECOMMENDATIONS. SUBSTITUTES MAY BE CONSIDERED; SUBMIT MANUFACTURER'S DATA PRIOR TO INSTALLATION.

EPOXY ADHESIVE ANCHORS

1. EPOXY ADHESIVE SHALL BE EPCON "CERAMIC 6" EPOXY MANUFACTURED BY ITW Ramset / Red Head. OR HIT RE 500 EPOXY ADHESIVE MANUFACTURED BY THE HILTI COMPANY. INSTALL PER MANUFACTURER'S RECOMMENDATIONS. SUBSTITUTES MAY BE CONSIDERED; SUBMIT MANUFACTURER'S DATA PRIOR TO INSTALLATION.

2. THREADED RODS SHALL BE ASTM A36. SIZES AND EMBEDMENT AS INDICATED ON THE DRAWINGS.

MASONRY

1. MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES (ACI 530.1/ASCE 6/TMS 602)" EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THESE CONTRACT DOCUMENTS.

2. COMPRESSIVE STRENGTH SHALL BE DETERMINED FOR EACH TYPE OF MASONRY BY THE UNIT STRENGTH METHOD.

   A. CONCRETE MASONRY: f'm = 1500 PSI AT 28 DAYS.

3. MATERIALS

   A. CONCRETE MASONRY UNITS: ASTM C90 TYPE I.

      1. BELOW GRADE: NORMAL WEIGHT AGGREGATE PER ASTM C33.

   B. FACING BRICK: ASTM C216 GRADE SW. COLOR AND SIZE AS NOTED ON THE ARCHITECTURAL DRAWINGS.

   C. MORTAR: ASTM C270 TYPE N

      1. PORTLAND CEMENT-LIME MORTAR:
         a. PORTLAND CEMENT: TYPE I.
         b. HYDRATED LIME: TYPE S.

   D. GROUT: ASTM C476. SLUMP 8" TO 10".

   E. REINFORCING STEEL: ASTM A615, 60 KSI YIELD.

   F. BRICK VENEER ANCHORS FOR WOOD STUD BACKUP: DUR-O-WAL D/A 213 OR WIRE-BOND RJ-711 WITH 3/16" DIAMETER PINTLE. HOT-DIPPED GALVANIZED PER ASTM A153 CLASS B. VERTICAL DISTANCE BETWEEN HORIZONTAL PINTLE WIRE AND CLIP PLATE SHALL NOT EXCEED 3/4 INCH. (FLAT CORRUGATED TIES ARE NOT PERMITTED.) SCREWS SHALL BE MINIMUM #10 SIZE AND SHALL BE CADMIUM-PLATED OR HOT-DIPPED GALVANIZED. (STAINLESS STEEL AND COPPER-COATED SCREWS ARE NOT PERMITTED.) PROVIDE BRICK VENEER ANCHORS WITH MAXIMUM HORIZONTAL SPACING OF 24" AND MAXIMUM VERTICAL SPACING OF 16". BRICK VENEER ANCHORS SHALL BE EMBEDDED 2" MINIMUM INTO BRICK.

4. KEEP AIR SPACE BEHIND VENEER FREE OF MORTAR DROPPINGS.

5. RUNNING BOND PATTERN SHALL BE USED FOR ALL MASONRY WORK UNLESS OTHERWISE NOTED.

6. PROVIDE MOVEMENT (CONTROL AND EXPANSION) JOINTS IN WALLS WHERE INDICATED ON ARCHITECTURAL DRAWINGS.

   A. MOVEMENT JOINTS IN BRICK: 3/8" WIDE CLEAN JOINT FILLED WITH EXPANSION JOINT MATERIAL PER ASTM D1056, CLASS RE 41. CAULK EXTERIOR FACE.

   B. PROVIDE BUILDING PAPER BOND BREAK BELOW LINTEL BEARING ADJACENT TO CONTROL JOINTS.

7. GROUT ALL CELLS BELOW GRADE SOLID.

STRUCTURAL STEEL

1. ALL DETAILING, FABRICATION, AND ERECTION SHALL CONFORM TO AISC SPECIFICATIONS FOR "DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", LATEST EDITION.

2. FIELD CONNECTIONS SHALL BE BOLTED EXCEPT WHERE WELDCONNECTIONS ARE INDICATED ON THE STRUCTURAL DRAWINGS.

3. WELDING SHALL BE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY (AWS D1.1).

4. MATERIALS:

   A. ROLLED WIDE FLANGE SHAPES UNLESS NOTED: ASTM A992 DUAL GRADE, Fy = 50 ksi.

   B. ROLLED OTHER SHAPES AND PLATES UNLESS NOTED OTHERWISE: ASTM A-36.

   C. TUBULAR SHAPES: ASTM A500, GRADE B.

   D. PIPE SHAPES: ASTM A53, TYPES E OR S GRADE B.

   E. BOLTS: ASTM A325-N, 3/4" DIAMETER UNLESS NOTED.

   F. ANCHOR RODS: ASTM F1554 - GRADE 36 KSI MATERIAL FULLY THREADED RODS HAVING A NUT TACK WELDED IN PLACE ON BOTTOM. MINIMUM EMBEDMENT AS NOTED ON THE DRAWINGS.

   G. FIELD WELDS: AWS E70XX, LOW HYDROGEN ELECTRODES.

   H. NON-SHRINK NON-METALLIC GROUT: CRD-C-621 AND ASTM C1107 FOR INTERIOR AND EXTERIOR APPLICATIONS.

5. PAINT AND PROTECTION:

   A. STRUCTURAL STEEL UNLESS NOTED: FABRICATOR'S STANDARD PRIME COAT. TOUCH UP AFTER ERECTION.

   B. MEMBERS TO BE ENCASED IN CONCRETE, SHALL HAVE NO PAINT.

   C. PROVIDE MINIMUM 3" CONCRETE COVER FOR ALL STEEL BELOW GRADE.

   D. LINTELS SUPPORTING EXTERIOR MASONRY WYTHES AND MEMBERS EXPOSED TO WEATHER IN FINISHED STRUCTURES: HOT DIP GALVANIZE PER ASTM A123 AFTER FABRICATION. COATING WEIGHT PER PARAGRAPH 5.1 OF ASTM A123 AND A153. FABRICATE ASSEMBLIES PER ASTM A143, A384, AND A385. TOUCH UP AFTER ERECTION WITH ORGANIC ZINC RICH PAINT COMPLYING WITH DOP-P-21035 OR MIL-P-26915, MULTIPLE COATS TO DRY FILM THICKNESS OF 8 MILS.

6. CONTRACTOR SHALL SUBMIT ERECTION AND SHOP DRAWINGS FOR REVIEW BY ENGINEER PRIOR TO FABRICATION BY ENGINEER PRIOR TO FABRICATION

WOOD

1. MATERIALS:

   A. FRAMING LUMBER:

      1. 2 x 8 AND LARGER: NO. 1 GRADE OR BETTER SOUTHERN PINE KILN DRIED.

      2. 2 x 4: STUD GRADE OR BETTER SPRUCE PINE FIR KILN DRIED.

      3. 2 x 6: NO. 2 GRADE OR BETTER SPRUCE PINE FIR KILN DRIED.

      4. CCA OR C2C PRESSURE TREAT PIECES IN CONTACT WITH FOUNDATION OR EXPOSED TO WEATHER.

   B. SHEATHING: 32/16 APA RATED ROOF SHEATHING EXPOSURE 1. 24/16 APA RATED STRUCTURAL WALL SHEATHING EXPOSURE 1. ALL SHEATHING TO BE NAILED WITH 8d NAILS AT 6" ON CENTER AT PANEL EDGES AND 12" ON CENTER AT INTERMEDIATE SUPPORTS UNLESS NOTED OTHERWISE. SEE. SHEET S120 FOR SPECIAL NAILING PATTERN FOR SHEAR WALLS.

   C. MANUFACTURED WOOD I JOISTS: SIZES AND SERIES AS SHOWN ON DRAWINGS AND AS MANUFACTURED BY TRUS JOIST CORPORATION. INSTALL PER MANUFACTURER'S RECOMMENDATIONS. MEMBERS BY OTHER MANUFACTURERS MAY NOT BE SUBSTITUTED WITHOUT PERMISSION OF THE ENGINEER.

2. UNLESS NOTED OTHERWISE, CONNECTORS SHALL BE MADE PER TABLE 2304.9.1 "RECOMMENDED FASTENING SCHEDULE", IN REFERENCED BUILDING CODE. STAPLES NOT PERMITTED FOR FASTENING APA RATED SHEATHING.

3. ALL CONNECTION HARDWARE SPECIFIED ON THE STRUCTURAL DRAWINGS SHALL BE MANUFACTURED BY THE SIMPSON STRONG-TIE COMPANY AND SHALL BE FASTENED AS SPECIFIED IN THE SIMPSON PRODUCT AND INSTRUCTION MANUAL.

4. FOR WOOD ROOF RAFTERS INSTALL ONE SIMPSON H5 HURRICANE TIE (UNLESS NOTED OTHERWISE) AT EACH MEMBER AT EACH BEARING LOCATION IN ADDITION TO THE TYPICAL NAILING REQUIREMENT IN THE "RECOMMENDED FASTENING SCHEDULE".

5. PROVIDE SOLID BLOCKING IN WALL CONSTRUCTION UNDER POSTS, MULTIPLE STUDS OR BEAM BEARINGS.

6. DOUBLE JOISTS SHALL BE PROVIDED BELOW ALL WALLS THAT RUN PARALLEL WITH THE JOISTS.

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NO.	DESCRIPTION	DATE
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GENERAL NOTES

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